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GENERAL REQUIREMENTS:

1. CONFLICTS: NOTES AND DETAILS ON THE DRAWINGS TAKE PRECEDENCE OVER THE GENERAL NOTES AND TYPICAL DETAILS IN CASE OF CONFLICT.
2. CODES: ALL MATERIALS AND WORK SHALL CONFORM TO THE REQUIREMENTS OF THE CALIFORNIA CODE OF REGULATIONS, TITLE 24, PART 2, 2019 CALIFORNIA BUILDING CODE (CBC).
3. SIMILAR WORK: WHERE CONSTRUCTION DETAILS ARE NOT SHOWN OR NOTED FOR ANY PART OF THE WORK, SUCH DETAILS SHALL BE THE SAME AS FOR SIMILAR WORK SHOWN ON THE DRAWINGS.
4. EXCAVATIONS: GENERAL CONTRACTOR TO LOCATE AND PROTECT UNDERGROUND OR CONCEALED CONDUIT, PLUMBING OR OTHER UTILITIES WHERE NEW WORK IS BEING PERFORMED.

DESIGN CRITERIA

1. DEAD LOADS:
   BLEACHERS (INCLUDES GIRDERS)..... 10 PSF
2. LIVE LOADS:
   LIVE LOAD..... 100 PSF
   SEATS..... 120 PLF
   FOOTBOARDS..... 100 PSF
   SWAY (PERPENDICULAR)..... 10 PLF
   SWAY (PARALLEL)..... 24 PLF
   GUARDRAILS AND HANDRAILS*..... 50 PLF
   * OR A 200 POUND CONCENTRATED LOAD APPLIED TO RAIL AT ANY POINT IN ANY DIRECTION.

   PRESS BOX:
   DEAD LOADS
   FLOOR..... 15 PSF
   ROOF..... 15 PSF
   WALL..... 6 PSF
   LIVE LOADS
   FLOOR/ROOF..... 100 PSF
3. LATERAL LOADS:

A) SEISMIC - SHORT PERIOD
SPECTRAL RESPONSE..... S_S = 1.640 g
ONE - SECOND PERIOD
SPECTRAL RESPONSE..... S_1 = 0.583 g
SITE CLASSIFICATION..... D
SITE COEFFICIENT..... Fa = 1.000
SITE COEFFICIENT..... Fv = 1.700
ADJUSTED SPECTRAL RESPONSE..... S_{MS} = 1.640 g
ADJUSTED SPECTRAL RESPONSE..... S_{M1} = 0.991 g
DESIGN SPECTRAL RESPONSE..... S_{DS} = 1.094g (2/3 S_{MS})
DESIGN SPECTRAL RESPONSE..... S_{D1} = 0.661 g (2/3 S_{M1})
RISK CATEGORY: III
I = 1.25
SEISMIC DESIGN CATEGORY..... D

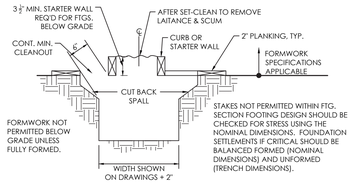
LATERAL SYSTEMS:

BOTH DIRECTIONS: CH. 15 ALL OTHER SELF-SUPPORTING STRUCTURES
R = 1.25
Rho = 1
Cs = 1.094 *W (1.0E longitudinal)
V = pCsW = 1.094W (1.0E strength)

A) WIND:
1. ULTIMATE DESIGN WIND SPEED, V_{ult} = 102mph
2. RISK CATEGORY = III
3. WIND EXPOSURE = C
4. APPLICABLE INTERNAL PRESSURE COEFFICIENT (GCpi)=0.55

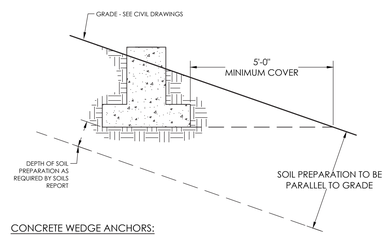
FOUNDATION:

1. GEOTECHNICAL INVESTIGATION: CONVERSE CONSULTANTS
   DATED: 10/24/19
   REPORT DATA: PROJECT No. 19-31-285-01
2. SOIL PRESSURES:
   SOIL BEARING..... 2000 psf
   COEFFICIENT OF FRICTION..... 0.3
   PASSIVE PRESSURE..... 150 pcf
3. SOIL REMOVAL AND RECOMPACTION: PER GEOTECHNICAL INVESTIGATION AND THE CONTRACT DOCUMENTS. SOILS WORK SHALL BE OBSERVED AND TESTED BY THE GEOTECHNICAL ENGINEER.
4. GEOTECHNICAL ENGINEER: SHALL OBSERVE FOOTINGS BEFORE PLACEMENT OF REINFORCING OR CONCRETE. FOOTING OBSERVATION AND COMPACTION REPORTS SHALL BE SENT TO THE ARCHITECT AND DSA.
5. SOIL PREPARATION: AS INDICATED IN THE GEOTECHNICAL INVESTIGATION REPORT AND AS SPECIFIED IN THE PROJECT SPECIFICATIONS. COORDINATE WITH GEOTECHNICAL ENGINEERS.
6. MANDATORY MINIMUM FORMWORK (unless fully formed).



A. STARTER WALL REQUIRED FOR ALL MASONRY OR CONCRETE WALLS.
B. FOUNDATION CONCRETE MAY BE PLACED DIRECTLY INTO NEAT EXCAVATIONS PROVIDED THE FOUNDATION TRENCH WALLS ARE STABLE AS DETERMINED BY THE ARCHITECT (STRUCTURAL ENGINEER) SUBJECT TO THE APPROVAL OF THE DIVISION OF THE STATE ARCHITECT. IN SUCH CASE THE MINIMUM FORMWORK SHOWN ON THE DRAWINGS IS MANDATORY TO INSURE CLEAN EXCAVATIONS IMMEDIATELY PRIOR TO AND DURING THE PLACING OF CONCRETE.

7. FOUNDATIONS GEOTECHNICAL / GRADING REQUIREMENTS:



CONCRETE WEDGE ANCHORS:

1. ANCHOR DIAMETER REFERS TO THE THREAD SIZE FOR THE WEDGE ANCHOR.
2. APPLY PROOF TEST LOADS TO WEDGE ANCHORS WITHOUT REMOVING THE NUT IF POSSIBLE. IF NOT, REMOVE NUT & INSTALL A THREADED COUPLER TO THE SAME TIGHTNESS OF THE ORIGINAL NUT USING A TORQUE WRENCH & APPLY LOAD.
3. REACTION LOADS FROM TEST FIXTURES MAY BE APPLIED CLOSE TO THE ANCHOR BEING TESTED, PROVIDED THE ANCHOR IS NOT RESTRAINED FROM WITHDRAWING BY THE FIXTURE(S).
4. TEST EQUIPMENT IS TO BE CALIBRATED BY AN APPROVED TESTING LABORATORY IN ACCORDANCE WITH STANDARD RECOGNIZED PROCEDURES.
5. THE FOLLOWING CRITERIA APPLY FOR THE ACCEPTANCE OF INSTALLED ANCHORS:

   HYDRAULIC RAM METHOD:
   THE ANCHOR SHOULD HAVE NO OBSERVABLE MOVEMENT AT THE APPLICABLE TEST LOAD. FOR WEDGE ANCHORS, A PRACTICAL WAY TO DETERMINE OBSERVABLE MOVEMENT IS THAT THE WASHER UNDER THE NUT BECOMES LOOSE.

   TORQUE WRENCH METHOD:
   WEDGE TYPE:
   THE APPLICABLE TEST TORQUE MUST BE REACHED WITHIN THE FOLLOWING LIMITS:

   ONE-HALF (1/2) TURN OF THE NUT;
   ONE-QUARTER (1/4) TURN OF THE NUT FOR THE 3/8 IN. SLEEVE ANCHOR ONLY.

6. TESTING SHOULD OCCUR 24 HOURS MINIMUM AFTER INSTALLATION OF THE SUBJECT ANCHORS.
7. TESTING VALUES:

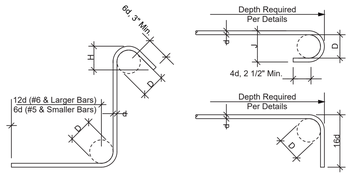
HILTI KB TZ ICC-ESR-1917 HARD ROCK TEST VALUES CONCRETE			
ANCHOR DIA. (IN.)	MINIMUM EMBEDMENT	ALLOWABLE TENSION LOAD (LBS.)	TORQUE (FT.-LBS.)
3/8	2 1/2"	2,560	25
1/2	3 1/4"	5,610	40
5/8	4 1/2"	9,300	60
3/4	5 1/2"	10,860	110

CONCRETE NOTES:

1. MAXIMUM SIZE AGGREGATE SHALL BE AS FOLLOWS:
   1 1/2"Ø
   AGGREGATE FOR CONCRETE SHALL CONFORM TO ASTM C33.
   GRADING OF AGGREGATE SHALL CONFORM TO TITLE 24, PART 2 CHAPTER 19-A, CBC.
2. MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS SHALL BE 3,000 PSI. SLUMP = 5" MAX. W/C RATIO IS 0.50 MAX.
3. ALL REINFORCING SHALL BE ASTM A615 GRADE 40 FOR #3, GRADE 60 FOR #4 AND LARGER. REINF. TO BE WELDED SHALL BE ASTM A706.
4. ALL DIMENSIONS SHOWN FOR LOCATION OF REINFORCING STEEL ARE TO FACE OF BAR AND DENOTE CLEAR COVERAGE. UNLESS SPECIFICALLY NOTED, CONCRETE COVERAGE SHALL BE AS FOLLOWS:
   3" FOR CONCRETE DEPOSITED DIRECTLY AGAINST GROUND (EXCEPT SLABS)
   2" FOR CONCRETE EXPOSED TO GROUND OR WEATHER BUT PLACED IN FORMS.
   PLACE REINF. AT MID-THICKNESS FOR SLABS ON GROUND.
5. CONCRETE SHALL NOT BE DROPPED THROUGH REINF. STEEL (AS IN WALL) SO TO CAUSE SEGREGATION OF AGGREGATES. IN SUCH CASES, HOPPERS AND VERTICAL CHUTES OR TRUNKS SHALL BE USED. CHUTES OR TRUNKS SHALL BE OF VARIABLE LENGTHS SO THAT FREE UNCONFINED FALL OF CONCRETE SHALL NOT EXCEED FIVE (5) FEET AND SUFFICIENT NUMBER SHALL BE USED TO INSURE THE CONCRETE BEING LEVEL AT ALL TIMES.
6. HORIZONTAL CONSTRUCTION JOINTS SHALL HAVE ENTIRE SURFACE REMOVED TO EXPOSED CLEAN AGGREGATE SOLIDLY EMBEDDED.
7. ALL STEEL COLUMN BASE PLATES AND STEEL BEAMS BEARING ON CONCRETE SHALL BEAR UPON 1" DRYPACK AND LEVELING NUTS EXCEPT AS NOTED OTHERWISE.
8. WHERE STEEL MEMBERS BEAR IN CONCRETE, GAPS BETWEEN BASE PLATE AND CONCRETE SHOULD BE DRY-PACKED WITH GROUT AFTER STEEL IS IN PLACE. GROUT SHALL BE PER CONCRETE CONTRACTOR AND SHALL BE 5 KSI MINIMUM.+
9. PROVIDE MINIMUM LAP SPLICES FOR CONTINUOUS REINFORCEMENT PER THE SCHEDULE PROVIDED BELOW. PROVIDE MINIMUM DEVELOPMENT FOR HOOKED BARS PER THE SCHEDULE BELOW.

TYPICAL LAP SPLICES AND DEVELOPMENT U.N.O. PER PLAN
3000 psi Conc., 60 ksi Rebar, 2" Clr. Min.

Bar Size	Hook Dev. Length (Ldh)	Lap Splice
#3	6"	17"
#4	8"	23"
#5	10"	28"
#6	12"	34"
#7	14"	49"



STANDARD TIES & STIRRUPS

Bar Size	D	H
#3	1 1/2"	2 1/2"
#4	2"	3"
#5	2 1/2"	3 3/4"
#6	4 1/2"	4 1/2"
#7	5 1/4"	5 1/4"
#8	6"	6"

STANDARD END HOOKS

Bar Size	D	J
#3	2 1/4"	3"
#4	3"	4"
#5	3 3/4"	5"
#6	4 1/2"	6"
#7	5 1/4"	7"
#8	6"	8"
#9	9 1/2"	11 3/4"
#10	10 3/4"	13 1/4"

NOTE: All bar bend diameters and end lengths must conform to the CRSI Manual of Standard Practice.

CONCRETE - EPOXY ANCHORED THREADED RODS:

1. MATERIALS: (ICC ESR 3187)
   HILTI HIT-HY200 EPOXY ADHESIVE
   CONCRETE (MIN)..... F'c = 3,000 PSI STONE AGGREGATE
   THREADED RODS..... ASTM A-153 TYPICAL
   NUTS..... ASTM A-563 HEX, GRADE A
2. INSTALLATION:
   A. CARBIDE-TIPPED DRILL BITS ANSI B212.15-1994.
   B. HOLES DRILLED WITH HOLLOW BIT, NO CLEANING OR BRUSHING REQUIRED.
   C. INSTALLATION AND ALLOWABLE LOADING:

HILTI HIT-HY200 HARD ROCK TEST VALUES CONCRETE								
ROD DIAM. (IN.)	BIT DIAM. (IN.)	MIN. EMBED. (IN.)	ALLOW. TENSION (LBS.)	LOAD SHEAR (LBS.)	MIN EDGE DIST. (IN.)	TIGHT. TORQUE (FT. LBS.)	TENSION TEST LOAD (LBS.)
3/8	7/16	3-3/8	1,370	1,320	5-1/4	18	2,700
1/2	9/16	4-1/2	2,400	2,375	6-3/8	30	4,800
5/8	3/4	5-5/8	3,595	3,375	7-1/2	75	7,200
3/4	7/8	6-3/4	5,025	4,600	10-1/8	150	10,050
7/8	1	7-7/8	5,340	5,330	11-5/8	175	10,070
1	1-1/8	9	6,440	6,140	13-1/2	235	13,000
1-1/4	1-3/8	11-1/4	8,645	8,875	14-1/2	400	17,300

3. CAPACITY LOADS ARE STRENGTH DESIGN LOADS.
4. DO NOT INSTALL IN CONCRETE THAT IS LESS THAN 7 DAYS OLD.
5. MINIMUM MEMBER THICKNESS TO RECEIVE ROD SHALL BE NO LESS THAN 1.5 TIMES THE ANCHOR EMBEDMENT DEPTH.
6. ANCHORS SHALL BE INSTALLED IN HAMMER DRILLED HOLES

STRUCTURAL STEEL:

1. CODES: AISC SPECIFICATION FOR STRUCTURAL STEEL FOR BUILDINGS; MANUAL OF STEEL CONSTRUCTION (14TH EDITION); STRUCTURAL WELDING CODE AWS D1.1 AND AWS D1.4. PIPES SHALL BE IDENTIFIED WITH MILL IDENTIFICATION IN ACCORDANCE WITH ASTM A-53 AND TUBE SHAPES IN ACCORDANCE WITH ASTM A-500.
2. IDENTIFICATION: ROLLED STRUCTURAL STEEL SHAPES SHALL BE IDENTIFIED WITH MILL IDENTIFICATION MARKS IN CONFORMANCE WITH ASTM A6.
3. MATERIALS:
   STRUCTURAL SHAPES
   WIDE FLANGE..... ASTM A992/A572, Fy = Gr. 50 (Fy = 50 ksi Min.), UNO
   CHANNELS AND ANGLES..... ASTM A529, Fy = Gr. 50 (Fy = 50 ksi Min.), UNO
   SWAYRODS..... ASTM A529, Fy = Gr. 50 (Fy = 50 ksi Min.), UNO
   HSS SHAPES (TUBE COLUMNS)..... ASTM A-500 GRADE B - 46 KSI
   PLATES ¼" THICK OR LESS..... ASTM A36 Gr. 36 (Fy = 36 ksi Min.), UNO
   PLATES GREATER THAN ¼" THICK..... ASTM A572 Gr. 50 (Fy = 50 ksi Min.), UNO
   BOLTS..... ASTM A-307, TYPICAL U.N.O (HOT DIP GALVANIZED)
   NUTS..... HEAVY HEX (HOT DIP GALVANIZED)
   NON-SHRINK GROUT..... ASTM C-1107 5,000 PSI (NON-METALLIC)
   ANCHOR BOLTS..... ASTM A36 OR ASTM A307 (Fy=36 ksi Min.) (HOT DIP GALVANIZED)
4. WELDING: ALL WELDING SHALL BE IN CONFORMANCE WITH AWS D1.1. WELDS ARE ALL AROUND WITH TYPE ER70S-6 WIRE MIG U.N.O.
5. ALL STEEL SHALL BE HOT DIP GALVANIZED TO CURRENT A.S.T.M. A-123.
6. ALL FIELD CONNECTIONS ARE NON-SLIP CRITICAL U.N.O. ALL CONNECTIONS ARE DESIGNED TO UTILIZE A307 BOLTS. IT IS ACCEPTABLE TO USE A325N BOLTS IN LIEU OF THE A307 BOLTS. THE INSTALLATION OF THESE BOLTS ARE TO BE TIGHTENED A SNUG TIGHT CONDITION AS SPECIFIED BY AISC.
7. SWAYROD THREADS SHALL BE PINGED WITH A HAMMER TO ELIMINATE REMOVAL OF NUT. AFTER FINAL TIGHTENING.

COLD FORMED MEMBERS (PRESS BOX):

MATERIALS:

STEEL PROPERTIES			
TUBE	4" SQUARE X 11 GA. TUBE	A500 GRADE A/B	Fy = 39ksi, MIN.
DIAGONAL ANGLE	L2 X 2 X 3/16"	A36 / A572 / A529	Fy = 50ksi, MIN.
BELLY PAN ANGLE	L2 X 2 X 1/8"	A36 / A572 / A529	Fy = 50ksi, MIN.
CEES	4 X 2 1/2 X 14 GA.	A1011 / A653	Fy = 33ksi, MIN.
FLAT STRAP	4" X 14 GA.	A1011 / A653	Fy = 33ksi, MIN.
ROOF PLATE	11GA. OR 1/8"	A1011 / A653 / A786	Fy = 27ksi, MIN.

1. CODES AND FABRICATION: STEEL STUD MANUFACTURER'S ASSOCIATION ICC ESR 3064P OR APPROVED EQUAL.
2. COLD FORMED SHAPES SHALL BE LIMITED TO A THICKNESS OF 11 GAGE OR THINNER.

ALUMINUM:

1. MATERIALS: ALUMINUM
   SHAPES.....ALLOY 6061-T6 OR 6005-T5 (Fy = 35 KSI)
   PLANKING.....ALLOY 6063-T6 (Fy = 25 KSI)
2. DISSIMILAR MATERIALS: WHERE ALUMINUM SURFACES ARE IN CONTACT WITH STEEL, THE STEEL SHALL BE GALVANIZED.
3. MILL FINISHED ALUMINUM WILL BECOME DISCOLORED DUE TO OXIDATION WHICH IS A NATURAL PHENOMENON & SHOULD BE EXPECTED.
4. ANODIZED ALUMINUM HANDRAIL IS 1 1/4" PIPE SIZE.
5. ALUMINUM TO CONFORM TO 2015 ALUMINUM DESIGN MANUAL.

SPECIAL INSPECTION/INSPECTOR REQUIREMENTS

REQUIREMENTS FOR SPECIAL INSPECTION:

1. PROJECT INSPECTOR: IN ACCORDANCE WITH TITLE 24, PART I, SECTIONS 4-333 AND 4-342.
2. CERTIFIED SPECIAL INSPECTOR: EMPLOYED BY THE DISTRICT AND APPROVED BY THE ARCHITECT, STRUCTURAL ENGINEER AND DSA.
3. REPORTS: PREPARED BY THE SPECIAL INSPECTOR AND SIGNED BY A CIVIL ENGINEER, SUBMITTED TO THE DSA, THE ARCHITECT, AND ENGINEER. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION; THEN, IF NOT CORRECTED, TO THE ARCHITECT, ENGINEER AND THE DSA (CBC 1704A.2.4).
4. THE SPECIAL INSPECTION IS TO BE CONTINUOUS DURING THE PERFORMANCE OF THE WORK UNLESS OTHERWISE SPECIFIED.

SUMMARY OF STRUCTURAL CONTINUOUS AND PERIODIC SPECIAL INSPECTION:

1. RESPONSIBILITY: IT IS THE RESPONSIBILITY OF THE PROJECT INSPECTOR TO INFORM THE SPECIAL INSPECTOR OR INSPECTION AGENCY AT LEAST ONE WORKING DAY PRIOR TO PERFORMING ANY WORK THAT REQUIRES SPECIAL INSPECTION.
2. SPECIAL INSPECTIONS:
   A) CONCRETE (CBC 1705A.3): DURING THE TAKING OF TEST SPECIMENS AND PLACING OF REINFORCED CONCRETE.
   B) BOLTS INSTALLED IN CONCRETE (CBC 1705A.3): PRIOR TO AND DURING THE PLACEMENT OF CONCRETE AROUND BOLTS.
   C) REINFORCING STEEL (CBC 1705A.3): DURING PLACING OF REINFORCING STEEL FOR ALL CONCRETE SPECIFIED TO HAVE SPECIAL INSPECTION.
   D) STRUCTURAL WELDING AND FABRICATION (CBC 1705A.2):
   i. DURING ALL SHOP AND FIELD WELDING IN ACCORDANCE WITH AWS D1.1 DURING SHOP FABRICATION.
   ii. WELDING INSPECTORS ARE TO BE AWS QC-1 CERTIFIED.
   a) ALL WELDING SHALL BE SPECIALLY INSPECTED BY AN AWS-CWI QUALIFIED INSPECTOR APPROVED BY DSA.
   iv. INSPECTION SHALL BE PER AWS D1.1, D1.3 OR D1.4 AND INCLUDE VERIFICATION THAT THE WPS IS BEING FOLLOWED.
   v. ALL STEEL AND WELDING MATERIALS SHALL BE IDENTIFIED AS REQUIRED BY THEIR ASTM OR AWS STANDARD.
   vi. ALL SUBMITTED TO THE OWNER REPRESENTATIVE BY THEIR WELDING INSPECTOR, NOT THE BLEACHER FABRICATOR.
   E) CONCRETE ANCHORS: FOR ALL THREADED ROD AND REINFORCING STEEL ANCHORED WITH EPOXY ADHESIVE IN CONCRETE: FOR ALL EXPANSION ANCHORS IN CONCRETE. INSPECTION / TESTING SHALL BE IN CONFORMANCE WITH THE ANCHOR MANUFACTURER'S EVALUATION REPORT AND SHALL INCLUDE AS A MINIMUM VERIFICATION OF HOLE DEPTH AND DIAMETER, CLEAN OUT, ALL MATERIALS, INSTALLATION TORQUE AND PROOF LOAD TESTS.

TAYLOR & SYFAN
Southern BLEACHER COMPANY
GRANDSTANDS • BLEACHERS • PADDIUMS
Established 1946
STRUCTURAL ENGINEERS
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SAN DIEGO | (858) 292-7300
553 S. Oak Knoll Ave. Pasadena, CA 91101
http://www.taylorsyfan.com
PO Box One, Graham, Texas 76450
1616 Eighth Street
Phone: 940.549.0753 Fax: 940.549.1365

REGISTERED PROFESSIONAL ENGINEER BRIAN T. DEAN S 6087 Exp. 06/30/21 STRUCTURAL STATE OF CALIFORNIA
S&S 12/03/20 PRINCIPAL

REV'T BY	DATE	DESCRIPTION
ES	12/03/20	NONE THIS PAGE
ES	08/11/20	NONE THIS PAGE
ES	07/14/20	DSA & DISTRICT COMMENTS
ES	02/26/20	NONE THIS PAGE

DATE 02/26/20 | DRAWN ES | CHK'D EMD/M4
JOB NUMBER 19333
SHEET B2 | OF B55

GENERAL NOTES

NORWALK - LA MIRADA UNIFIED SCHOOL DISTRICT
LA MIRADA HIGH SCHOOL
LA MIRADA, CALIFORNIA

22" X 34" (1 1x17 = 1/2 indicated scale)

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 03-120551 INC:
REVIEWED FOR
SS ☑ FLS ☑ ACS ☑
DATE: 01/11/2021

REVISIONS

1 DSA_V2 Date 1

LICENSED ARCHITECT HALENA L. JUBANY Ren. 5-31-21 STATE OF CALIFORNIA

NORWALK LA MIRADA UNIFIED SCHOOL DISTRCT
LA MIRADA HIGH SCHOOL NEW FOOTBALL STADIUM PROJECT
13520 ADELFA DRIVE, LA MIRADA, CA. 90638

DSA # 03-120551

NAC ARCHITECTURE
NAC NO. 161-19015
DATE 12/10/2020

GENERAL NOTES

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